

LONG ISLAND BASIN

LONG POND DAM NO. 3

WESTCHESTER COUNTY, NEW YORK **INVENTORY NO. N.Y. 115** 

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

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**NEW YORK DISTRICT CORPS OF ENGINEERS** 

SEPTEMBER 1981



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The classification of "unsafe" applied to a dam because of a "seriously inadequate spillway" is not meant to connote the same degrectof emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean, however, that based on an initial screening, and preliminary computations, there appears to be a serious deficiency in spillway capacity so that if a severe storm were to occur, overtopping and failure of the dam would take place, significantly increasing the hazard to loss of life downstream from the dam.

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# LONG ISLAND BASIN

# LONG POND DAM NO. 3

WESTCHESTER COUNTY, NEW YORK INVENTORY NO. N.Y. 115

# PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM



NEW YORK DISTRICT CORPS OF ENGINEERS

SEPTEMBER 1981

# PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D. C., 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigations, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

# PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM LONG POND DAM NO. 3 I.D. NO. N.Y. 00115 D.E.C. NO. LONG ISLAND BASIN WESTCHESTER COUNTY, NEW YORK

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# PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

NAME OF DAM:

Long Pond Dam No. 3, N.Y. 00115

STATE LOCATED:

New York

COUNTY LOCATED:

Westchester

STREAM:

Mianus River

BASIN:

Long Island

DATE OF INSPECTION:

June 9, 1981

#### ASSESSMENT

Examination of available documents and a visual inspection of the dam and the appurtenant structures did not reveal conditions which constitute an immediate hazard to human life or property.

Using Corps of Engineers' Screening Criteria, it has been determined that the dam would be overtopped for all storms exceeding approximately 41 percent of Probable Maximum Flood (PMF). The service spillway is, therefore, adjudged as "seriously inadequate" and the dam is assessed as unsafe, non-emergency.

The classification of "unsafe" applied to a dam because of a "seriously inadequate spillway" is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean, however, that based on an initial screening, and preliminary computations, there appears to be a serious deficiency in spillway capacity so that if a severe storm were to occur, overtopping and failure of the dam would take place, significantly increasing the hazard to loss of life downs ream from the dam.

It is, therefore, recommended that within 3 months of notification to the owner, detailed hydrological-hydraulic investigations of the structure should be undertaken to more accurately determine the site specific characteristics of the watershed and their affect upon the overtopping potential of the dam. At the same time a dam break analysis should be carried out to ascertain the effect of a sudden flood from the upstream reservoir discharging into the lower reservoir. The results of these investigations will determine the appropriate remedial measures which will be required to achieve a spillway capacity

adequate to discharge the outflow from at least the & PMF.

In the interim, a detailed emergency action plan must be developed and implemented during periods of unusually heavy precipitation. Also, around-the-clock surveillance of the structure must be provided during these periods.

The following remedial measures must be completed within one year:

- 1. Nature of the seepage at the central toe of the dam should be investigated.
- Location of the reservoir drain discharge should be determined.
- 3. Only after locating the reservoir drain discharge, the workability of the drain valve should be evaluated. If found to be inoperable or defective, it should be repaired to a good working condition or replaced.
- 4. The concrete on the spillway training walls and downstream bottom slab should be repaired.
- 5. The rocklined spillway discharge channel should be evaluated to determine if it has sufficient capacity to contain calculated maximum spillway flows.
- 6. The seepage area at the south abutment should be blanketed with a properly filtered drainage material to allow controlled transportation of seepage waters downslope in addition to minimizing the potential for piping of finer grained embankment soils.
- 7. All small trees, dead large trees and large trees located near the crest and upstream slope should be cut down and removed. Larger trees on the downstream slope should be inventoried and their condition monitored. If one of these trees dies it should be cut down and the area around the stump should be monitored for the development of seepage. Holes and depressions developing as a result of tree removal should be backfilled and sealed.
- 8. The low lying area, upstream of Duck Pond immediately adjacent to the dam's toe should be backfilled to prohibit backwater encroachment on the downstream slope.
- 9. The upstream slope of the dam should be properly protected against erosion.

10. A program of periodic inspection and maintenance of the dam and appurtenances including yearly operation and lubrication of the repaired gates should be provided. This program should be documented for future reference. The emergency action plan, described in Section 7.1d, should be maintained and updated periodically during the life of the structure.

Eugene O'Brien, P.E. New York No. 29823

Approved By:

Col. W. M. Smith, Jr.

New York District Engineer

Date:

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GENERAL OVERVIEW OF DAM

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PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
LONG POND DAM NO. 3
I.D. NO. N.Y. 00115
D.E.C. NO.
LONG ISLAND BASIN
WESTCHESTER COUNTY, NEW YORK

SECTION 1 - PROJECT INFORMATION

# 1.1 GENERAL

# a. Authority

The Phase I inspection reported herein was authorized by the Department of the Army, New York District, Corps of Engineers by Contract No. DACW 51-81-C-0008, dated 14 December 1980, in fulfillment of the requirements of the National Dam Inspection Act, Public Law 92-367, 8 August 1972.

# b. Purpose of Inspection

The inspection was conducted to evaluate the existing conditions of the dam, to identify deficiencies and hazardous conditions, to determine if these deficiencies constitute hazards to life or property, and to recommend remedial measures where required.

#### 1.2 DESCRIPTION OF THE PROJECT

#### a. Description of Dam and Appurtenant Structures

The Long Pond Dam No. 3 is composed of a 355 ft long earth and rockfill embankment having a maximum height of approximately 40 ft and crest width of 34 ft. Upstream slopes of the dam typically range from 1 Vertical to 5 Horizontal (1V:5H) to 1V:6H. In the vicinity of the spillway approach upstream slopes are steeper, on the order of 1V:3H. No slope protection is present on the upstream face. The downstream slope typically has a 1V:1.5-1.0H slope descending about 20 ft from the crest to a 20 ft wide horizontal bench and then continuing

downslope at a 1V:1.5-2.0H slope to the toe. Near the north abutment in the vicinity of the rocklined spillway channel, downstream slopes are typically 1V:3H. Typical slope geometry is shown in section view in Appendix A.

The spillway, located at the north abutment contact in a reinforced concrete box culvert 4 ft high and 6 ft wide. Base slab elevation of the spillway is about 6 ft below the dam crest. The spillway is uncontrolled and passes water through the dam crest discharging into a rocklined channel running down the north abutment.

The dam which creates a recreational lake is equipped with a reservoir drain of unknown dimension and construction. Gatehouse for this drain is located about 100 ft upstream of the center of the dam (See Photograph 6).

North Lake Dam is situated approximately 1,200 ft upstream of Long Pond Dam. An uncontrolled overflow spillway passes water from North Lake directly into Long Pond.

# b. Location

Long Pond No. 3 Dam is located at the south end of Long Pond Court within the Windmill Farms Development, approximately 6½ miles northeast of the City of North Castle, West-chester County, New York.

# c. Size Classification

The dam is 40 ft high and has a reservoir at this height with a storage capacity of approximately 115 acre-feet and is therefore classified as a small dam.

# d. Hazard Classification

The dam is in the "high" hazard potential category because it is located upstream of a moderately to densely populated area of the Windmill Farms subdivision.

# e. Ownership

The Long Pond Dam No. 3 is located on property owned by Mr. K. Karl Mueller, 4 Long Pond Court, Windmill Farms, Armonk, N.Y. 10504, Telephone No.: (914) 273-8074. However, the lake bottom property and the reservoir drain control house is owned by the Town of New Castle.

# f. Purpose of Dam

The dam was constructed to create a lake used for recreational purposes.

# g. Design and Construction History

Design drawings or construction records do not exist for the Long Pond Dam No.3. The dam was reportedly designed by Elwyn E. Seelye and Company, Park Avenue, New York, N.Y. and built circa 1936.

# h. Normal Operating Procedure

Water periodically flows through the uncontrolled spillway. It is not known if the reservoir drain is periodically used for water level control or even operational.

#### 1.3 PERTINENT DATA

a.	<u>Drainage Area</u> (sq. miles)	0.46
b.	Discharge at Dam Site (cfs)	
	Ungated Spillway at Maximum Pool	222
	Capacity of Reservoir drain	unknown
	Total Discharge, Maximum Pool(El. 476)	222 <u>+</u>
c.	Elevation (ft above MSL USGS Datum) *	
	Top of Dam	476 <u>+</u>
	Maximum Design Pool	474 <u>+</u>
	Spillway, Upstream Invert	470 <u>+</u>
	Inverts. Reservoir Drain	unknown

<sup>\*</sup> All elevations are based on pond level datum relative to an elevation interpolated from the USGS QUAD sheet.

d.	Reservoir	
	Length of Maximum Pool (miles)	0.096
	Length of Shoreline at Spillway	7
	Crest (miles)	0.74
	Surface Area (acres)	11.28
e.	Storage (acre-feet)	
	Reservoir at Spillway Crest	115
	Reservoir at Maximum Pool	173.6
f.	Dam	
	Туре	Earth and Rock Fill
	Length (feet)	355 ft
	Height (feet)	40
	Upstream Slope	1V:5H
	Downstream Slope	1V:1.5-2.0H w/
		20 ft bench at mid-
		height _
	Crest Elevation (feet)	476
	Crest Width (feet)	34
	Cutoff Type	None
	Grout Curtain	None
g.	Spillway	
	Туре	Concrete, 4' x 6' box

Type Concrete, 4' x 6' box culvert with rocklined open channel running down north abutment

Length (feet) 54 Crest Elevation (feet) 470\*

# h. Reservoir Drain and Pipeline

A control structure, located approximately at the center of the dam, and about 100 ft upstream, houses the control valves for the reservoir drain. Valve type, pipe size and

<sup>\*</sup> See Note on Page 3.

location of discharge are unknown, and/or could not accurately be determined at the time of inspection. Access to the offshore gate house was not possible during inspection, therefore, the condition or workability of the drain control could not be ascertained.

# 2.1 GEOLOGY

The records of the owner contain no data on site geology. However, there is data available in the published literature (Ref.4) on the general geology of the area. Long Pond Dam. No.3 is located in the New England Upland Section of the New England Maritime Physiographic Province. The bedrock in this section consists of metamorphic, igneous and sedimentary rocks which have undergone a complex sequence of deposition, folding, faulting and erosion. In the vicinity of the damsite, bedrock is primarily composed of Fordham gneiss.

# 2.2 SUBSURFACE INVESTIGATION

There is no record of subsurface investigation for the dam. The shallow surface soils in the area are of glacial origin and for the most part consist of complex sands, silts, gravels and boulders.

# 2.3 DAM AND APPURTENANT STRUCTURES

The files of the owner contain no prints, design data or construction record of the dam. There are no drawings of the outlet works or any subsequent changes which may have been made.

# 2.4 CONSTRUCTION RECORDS

No information has been located in relation to the construction of the dam, spillway or outlet works. Based on the construction history of several dams built in conjunction with development of the Windmill Farms Area, the completion of the project was probably circa 1936.

#### 2.5 OPERATION RECORDS

According to the owner there are no records of operation for the dam.

#### 2.6 EVALUATION OF DATA

The data available in the records of the owner, along with a field inspection and personal interviews are sufficient to support a Phase I evaluation of the dam.

#### SECTION 3 - VISUAL INSPECTION

#### 3.1 FINDINGS

# a. General

The visual inspection of the Long Pond Dam No. 3 was performed on June 9, 1981. The weather was overcast with intermittent showers and temperatures ranges in the mid to high 70's F. The reservoir was at a level approximately 5.5 ft below the crest of the dam, about 6 inches deep in the spillway.

#### b. Dam

The dam is generally in good condition, showing no noticeable signs of vertical or horizontal displacements. Slopes appear to be stable. Seepage at the south abutment contact at a point mid-height on the slope was observed. Quantities of seepage were relatively small and soil transport did not appear to be occurring; however, this seepage still requires immediate attention. There is no slope protection on the upstream face of the dam, however, no significant erosion was observed.

Additionally, the following adverse conditions were noted:

- 1. There are a great number of very large trees growing on the downstream slope of the dam. Near the crest of the dam the root systems are likely to extend to the upstream face of the dam. This could result in the formation of seepage paths through the dam.
- 2. There is no emergency action plan for the project.

#### c. Spillway

The reinforced box culvert spillway, located at the north abutment, is in fair condition. Minor spalling of the concrete approach walls was observed. Of major concern, is the

broken and displaced slab at the downstream end of the spillway. A majority of the water passing through the spillway presently passes and exits below the downstream slab which, as a result is severely undermined (See Photograph 8). The spillway channel located along the north abutment is heavily revetted, with massive boulders (See Photograph 9) and appears in good condition for the present amount of overflow. There is no definite shape or rigid alignment to the channel and hence it may tend to extend onto unprotected portions of the embankment when required to handle large quantities of flow.

# d. Appurtenant Structures

Condition, description and workability of the upstream reservoir drain controls are not known. Location of the downstream exit of the reservoir drain could not be positively identified, but is suspected to be partially buried within a pile of rock near the central downstream toe of the dam. Flowing water from this area may indicate that the valve is partially open or leaking.

# e. Downstream Channel

The spillway tailrace channel passes along the north abutment of the dam and exits in the natural channel of the Mianus River.

# f. Abutments

The abutment-dam contact areas are generally in good condition with the exception of a small seep located about mid-slope at the south abutment.

#### q. Reservoir Area

In the general vicinity of the dam and along the south of the lake, reservoir slopes are generally stable. No slides, rock falls or sloughing was observed during the inspection. There are however, two unusual conditions in the reservoir area which are considered significant. These are:

- embankment, forms an estimated 600 to 800 ft long stretch of reservoir shoreline at the southeast end of the lake. At its present level, the lake does not immediately infringe on the embankment, however, during flood stage, this portion of the roadway embankment will act as a dam. Based on "rough" measure—ments made during the dam inspection visit, it is estimated that a majority of this 600 to 800 ft long stretch of Long Pond road is 1 to 1½ ft lower in elevation than the crest of Long Pond Dam No. 3 and consequently may act as a secondary spillway under severe flood conditions.
- 2. The close proximity of North Lake Dam is located immediately upstream of the reservoir.

# 3.2 EVALUATION OF OBSERVATIONS

Although deficiencies were observed, there is no indication that the dam is in imminent danger. Some of the deficiencies observed in the previous paragraphs are minor and should be corrected by the owners' maintenance forces. Other conditions described above, however, represent conditions which may have potential for further deterioration and for this reason these conditions need to be further investigated and corrected.

The following is a summary of the problem areas encountered requiring further investigations and corrective action. An appropriate recommended action is included.

- 1) Nature of the flowing water emitting from the rockpile at the central toe area of the dam should determined. This may be the location of the reservoir drain outlet.
- 2) Condition and workability of the reservoir drain controls should be evaluated.
- 3) All small trees (trunk less than 8 inches in diameter), large dead trees and large trees located near the crest of the dam should be removed with the resulting holes or depressions properly backfilled and seeded. Larger trees on the downstream slope should be inventoried and their conditions monitored.

- 4) Seepage area at the south abutment should be blanketed with a graded gravel to mitigate surface water erosion or potential piping. This area should also be monitored on a frequent basis.
- 5) The downstream end of the spillway should be repaired including backfilling undercut areas.
- 6) A program of periodic inspection and maintenance of the dam and appurtenances, including yearly operation and lubrication of the moving parts of the reservoir drain should be provided. This program should be documented for future reference. The aforementioned emergency action plan should be maintained and updated periodically during the life of the structure.

# SECTION 4 - OPERATION AND MAINTENANCE PROCEDURES

# 4.1 PROCEDURES

No written operation and maintenance procedures exist for the facility. The normal operation is to allow uncontrolled flow through the spillway.

# 4.2 MAINTENANCE OF DAM

It is reported that no routine maintenance of the dam is performed.

# 4.3 WARNING SYSTEM IN EFFECT

No warning system is in effect or in preparation.

# 4.4 EVALUATION

The overall operation and maintenance of the Long Pond Dam No. 3 is considered inadequate as a result of the following conditions:

- Inaccessible or otherwise buried reservoir drain discharge
- 2. Deteriorating spillway
- 3. Uncontrolled seepage at the south abutment
- 4. Absence of a written operation and maintenance procedure
- 5. Absence of any written maintenance history

# 5.1 DRAINAGE AND CHARACTERISTICS

The Long Pond Dam is located in North Castle Township, Westchester County, New York (Hydrologic Unit Code No. 01100006). The area contributing to Long Pond is 0.52 sq. miles, and includes tow reservoirs, North Lake and Windmill Lake. The basin is hilly with steep wooded slopes and narrow valleys. North Lake controls 0.26 sq. miles of the drainage area, while Windmill Lake controls 0.06 sq. miles (40.4 acres).

# 5.2 ANALYSIS CRITERIA

Spillway capacity adequacy was analysed by developing a design flood, using the unit hydrograph and the Probable Maximum Precipitation (PMP). For the analyses, the drainage area was ivided into four sub-areas. The runoff from the drainage area contributing to Windmill Lake was assumed to be totally controlled by the available storage of the lake and was not included. However, the watershed downstream of Windmill Lake - subarea 2 - was assumed to contribute to Long Pond. Runoff from the North Lake sub-area was routed through the reservoir, then channel routed flow was sub-sequently combined with the runoff calculated for the Long Pond drainage area (Subarea 1) to form a composite hydrograph. The computed runoff hydrograph, Subarea 2, was then combined with the hydrograph from the two previous subareas and routed through Long Pond.

The all seasons 200 sq. miles 24 hours PMP for the Long Pond area, taken from Weather Bureau sources (Ref. 2) is 22 inches. The three unit hydrographs were computed by the Snyder method using coefficients of 2 and 0.6 for  $C_{\rm T}$  and  $C_{\rm p}$ , respectively. The inflow hydrographs were developed by the U.S. Army Corps of Engineers HEC-1DB computer program (Ref. 1). Loss rates of 1.0 inch initial loss ad 0.1 inch/hour constant loss were estimated as representative of the basin for the design storm.

In accordance with the recommended guidelines for Safety and Inspection of Dams (Ref. 3), the adequacy of the spillway was analyzed using the Probable Maximum Flood (PMF). A multiplan analysis was performed for the full 0.75, 0.50 and 0.25 PMF for tow cases.

The Case I analysis considers discharge from the Long Pond reservoir during flood stage to be routed solely through the spillway located at the north abutment of the dam with the excess being permitted to overtop the structure.

Case II analysis considers the possibility that a portion of the highway embankment bounding the southeast end of the lake has a crest elevation approximately 1 ft lower than Long Pond dam and therefore would act as a secondary spillway in combination with the service spillway during a PMP event.

# 5.3 SPILLWAY CAPACITY

The service spillway located near the north abutment of Lond Pond dam is a reinforced concrete box culvert 6.0 ft wide by 4.0 ft high, with an invert elevation estimated to be at 470 ft (MSL). The maximum computed discharge for a lake water surface at elevation 476 (top of dam) is 222 cfs.

The hypothesized secondary spillway provided by the low lying section of highway embankment at the southeast end of the lake has an estimated invert elevation of 475 (MSL) and an estimated trapezoidal section having a 250 ft base width with side slopes of approximately 0.5 percent. Routed flow would be over the road and routed north and south to two adjacent unnamed drainage areas. The computed maximum discharge of the "secondary spillway" for a lake level at 476 (MSL) is 669 cfs.

# 5.4 RESERVOIR CAPACITY

The normal reservoir capacity is listed as 115 acre-feet. The computed surcharge storage of 58.6 acre-feet is equivalent to approximately 2.4 inches of runoff over the entire basin.

# 5.5 FLOODS OF RECORD

There are no records available of Floods or maximum lake elevations.

# 5.6 OVERTOPPING POTENTIAL

The potential of the dam being overtopped was investigated on the basis of the spillway or spillways discharge capacities and the available surcharge storage to meet the selected design flood inflows for each of the two cases described previously.

The analyses were performed assuming that the water surface in the reservoir was at the service spillway crest elevation (El 470) at the start of the flood event. The computed PMF peak inflow was 1,647 cfs. The HEC-1DB analysis indicated that the dam would be overtopped by all floods exceeding 41 percent of the PMF for Case I. Table 1 is a summary of the computer analysis.

Table 1
Results of Case I Analysis

RATIO OF PMF	PEAK INFLOW (cfs)	PEAK OUTFLOW (cfs)	OVER- TOPPING (feet)
1.00	1,647	1,630	1.14
0.75	1,224	1,214	0.90
0.50	806	761	0.60
0.25	242	159	0.00

For Case II the dam would be overtopped by all floods equal to an exceeding 58 percent of the PMF. The results of this analysis are presented in Table 2.

Table 2
Results of Case II Analysis

RATIO OF PMF	PEAK INFLOW (cfs)	PEAK OUTFLOW (cfs)	OVER- TOPPING (feet)
1.00	1,647	1,628	0.22
0.75	1,224	1,222	0.11
0.50	806	847	0.00
0.25	242	159	0.55

#### 5.6 EVALUATION

The service spillway of the Long Pond dam is incapable of passing flows exceeding 41 percent of the PMF without the dam being overtopped. When considering the low roadway embankment at the southeastern corner of the reservoir (approximately 1,500 ft south of the dam) acting as a secondary spillway floods less than or equal to 50 percent of the PMF would not overtop the dam. Because of the uncertainty of the data regarding the "secondary spillway", the service spillway must be assessed as being seriously inadequate.

# 6.1 EVALUATION OF STRUCTURAL STABILITY

# a. Visual Observations

Visual observations did not reveal any conditions which at present adversely affect the structural stability of the dam. Seepage observed at the south abutment and flowing water, observed at the central toe of the dam did not show transportation of any soil particles and is therefore not considered an immediate hazard. The source of the flowing water at the toe, however, does require further investigation. Stability of the spillway structure appears adequate provided the downstream portions of the cracked and undercut slab are properly repaired. Location of the downstream reservoir drain discharge needs to be determined.

# b. Design and Construction Drawings

There are no design or construction drawings in existence to the best knowledge of the owner.

# c. Stability Analysis

Based on the results of visual observations, the stability of the dam appears adequate for a Phase I assessment. Similarly, stability of the service spillway is also judged as adequate.

# d. Operating Records

There are no operating records for the dam.

# e. Post-Construction Changes

There are reportedly no post-construction changes to the dam.

# f. Seismicity Stability.

The dam is located in Seismic Zone 1 and in accordance with recommended Phase I guidelines. However, based on the past earthquake activity in the region, the New York Geological Survey considers the site to be more characteristic of a Zone 2 Setting. Based on this assessment, the dam is considered in the Seismic Zone 2.

#### SECTION 7 - ASSESSMENT/RECOMMENDATIONS

#### 7.1 ASSESSMENT

# a. Safety

Examination of available documents and visual inspection of the dam and the appurtenant structures did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.

The earth embankment is considered to be stable under present operating conditions; however, poor condition of the downstream spillway slab, associated undercutting, and seepage observed at the south abutment may present a hazard condition under severe flood stage conditions. The stability of the dam is further endangered by the presence of a large masonry dam of unknown stability located immediately upstream of the reservoir.

Using the Corps of Engineers' Screening Criteria for review of spillway adequacy, it has been determined that the dam would be overtopped for all storms exceeding approximately 41 and 58 percent of the PMF for Cases I and II Analyses, as described in Section 5.2. The overtopping of the dam could cause the erosion of both abutments and the downstream face of the dam resulting in a breach, thus significantly increasing the hazard for loss of life downstream. The service spillway is therefore assessed as unsafe, non-emergency.

The classification of "unsafe" applied to a dam because of a "seriously inadequate" spillway is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean, however that based on an initial screening and preliminary computations, there appears to be a serious deficiency in spillway capacity so that if a severe storm were to occur, overtopping and failure of the dam would take place, significantly increasing the hazard to loss of life downstream from the dam.

# b. Adequacy of Information

This report and its conclusions are based on visual inspection, interviews, and office hydrologic/hydraulic studies. This information and data are adequate for a Phase I inspection.

# Need for Additional Investigations

Since the spillway is considered to be "seriously inadequate", additional hydrologic/hydraulic investigations are required to more accurately determine the site specific characteristics of the watershed including detailed information of the roadway embankment assumed to act as a secondary spillway for Case II analyses. After the in-depth hydrologic/hydraulic investigations have been completed, remedial measures must be initiated to provide spillway capacity sufficient to safely discharge the outflow from the ½ PMF event.

Additionally it is recommended that a dambreak type study be performed to assess the effect of the upstream North Lake reservoir on the safety of the Long Pond Dam.

# d. Urgency

The additional hydrologic/hydraulic investigations which are required must be initiated within 3 months from the date of notification. Within 12 months of notification, remedial measures as a result of these investigations must be initiated, with completion of these measures during the following year. In the interim, an emergency action plan for the notification of downstream residents and proper governmental authorities in the event of overtopping should be developed, and around-the-clock surveillance of the dam during periods of extreme runoff should be provided. The other problem areas listed below must be corrected within one year from notification.

# 7.2 RECOMMENDED MEASURES

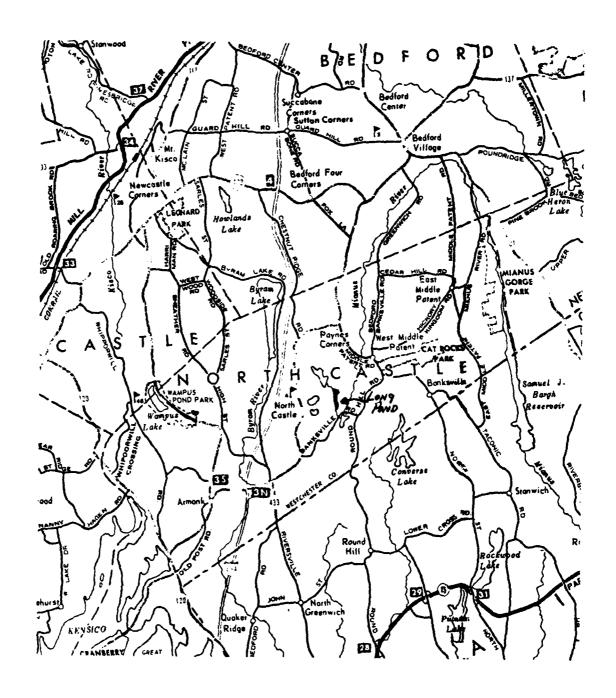
Recommended measures are as follows:

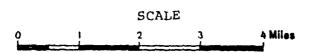
1. Nature of the seepage at the central toe of the dam should be investigated.

- 2. Location of the reservoir drain discharge should be determined.
- 3. Only after locating the reservoir drain discharge, the workability of the drain valve should be evaluated. If found to be inoperable or defective it should be repaired to a good working condition or replaced.
- 4. The concrete on the spillway training walls and downstream bottom slab should be repaired with proper backfilling of the unsupported portion of the slab being performed as part of this repair.
- 5. The rocklined spillway discharge channel should be evaluated to determine if it has sufficient capacity to contain calculated maximum spillway flows.
- 6. The seepage area at the south abutment should be blanketed with a properly filtered drainage blanket to allow controlled transportation of seepage waters downslope, in addition to minimizing the potential for piping of finer grained embankment soils.
- 7. All small trees, dead large trees and larger trees located near the crest should be cut down and removed. Larger trees on the downstream slope should be inventoried and their conditions and the area around the stump should be monitored for the development of seepage. Holes and depressions resulting from removal of trees should be backfilled and sealed.
- 8. The low lying area upstream of Duck Pond immediately adjacent to the toe of the dam should be backfilled to prohibit backwater encroachment on the downstream slope.
- 9. The upstream slope of the dam should be provided with proper slope protection against erosion.
- 10. A program of periodic inspection and maintenance of the dam and appurtenances including yearly operation and lubrication of the repaired gates should be provided. This program should be documented for future reference. The emergency action plan, described in Section 7.1d, should be maintained and updated periodically during the life of the structure.

DRAWINGS

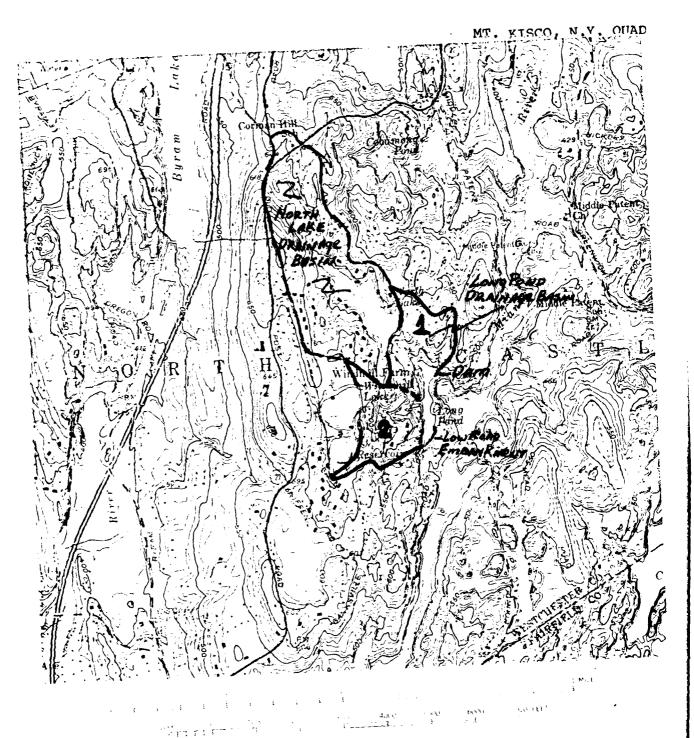
APPENDIX A





VICINITY MAP

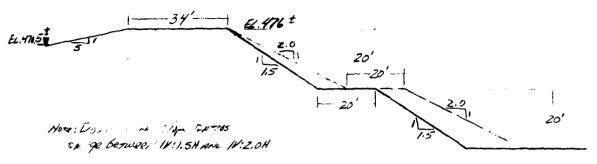
LONG POND DAM NO. 3



SCALE 1:24000

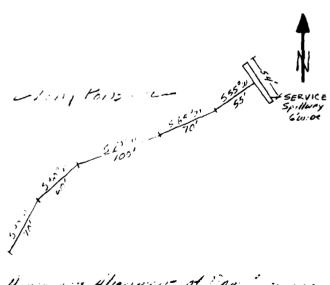
TOPOGRAPHIC MAP
LONG POND DAM NO. 3

Job No. 1579-10	Sheet of
Project	Date <u>6 26 8/</u>
Subject	By JFW
1 CREST Alignment	Ch'k. by



Appreximate Maximum Dane Com contents

Scrike 1"=30"



Hoper were Alignment of Dan In see

PHOTOGRAPHS .

APPENDIX, B



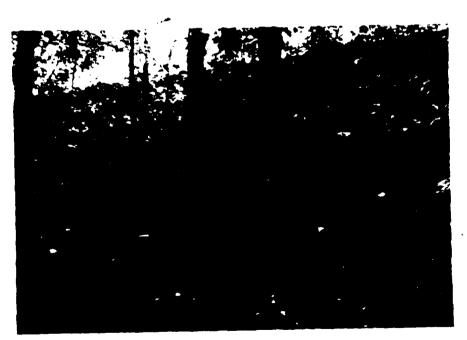
2. VIEW FROM NORTH END OF CREST LOOKING SOUTH DOWN CENTER LINE OF DAM.



3. VIEW OF UPSTREAM SLOPE FROM NEAR SPILLWAY ENTRANCE.



4. VIEW OF DOWNSTREAM SLOPE
AS SEEN FROM
TOE. (NOTE:
BENCHED SLOPE
GEOMETRY)



5. VIEW OF DOWNSTREAM SLOPE AS SEEN FROM MID SLOPE.



6. RESERVOIR DRAIN GATE HOUSE



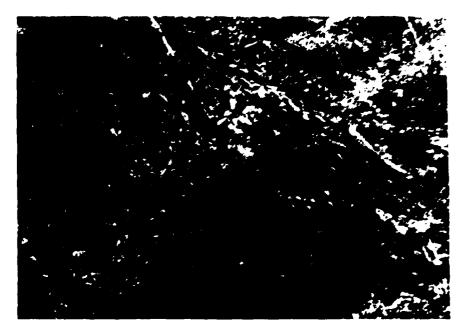
7. UPSTREAM VIEW OF SPILLWAY. (NOTE: SPALLING OF CONCRETE ON TRAINING WALLS).



8. CRACKED AND UNDERMINED SLAB AT DOWN-STREAM END OF SPILLWAY.



9. ROCK LINED SPILLWAY DISCHARGE CHANNEL, DOWNSTREAM NEAR TOF OF SLOPE.



10. OVERVIEW OF SEEPAGE AT CONTACT ON SOUTH ABUTMENT.



11. CLOSEUP VIEW
OF SEEPAGE AREA
REFERENCED
ABOVE.



12. DISCHARGE AT CENTER TOE OF DOWNSTREAM SLOPE. PRESUMABLE BURIED RESERVOIR DRAIN OUTLET.



13. OVERVIEW OF LOW LYING DUCK POND BACKWATER AREA ADJACENT TO TOE OF DOWNSTREAM SLOPE.

VISUAL INSPECTION CHECKLIST

#### VISUAL INSPECTION CHECKLIST

)	Bas	ic Data
	a.	General
		Name of Dam Long Pond Dam No. 3
		Fed. I.D. # <u>NY 00 115</u> DEC Dam No. /
		River Basin Manys River
		Location: Town North Castle County Westchestee
		Stream Name Univanies Drawage
	·	Tributary of Miamus River
		Latitude (N) 41-08.5 Longitude (W) 073-40.4
		Type of Dam Erry Rido Rockfill
		Hazard Category HIGH (1)
		Date(s) of Inspection 9 June 1981
		Weather Conditions Ovaring rimer interior houses
	•	Reservoir Level at Time of Inspection 470.5= (ESTIMATED BALLOW MS65 MA)
	<b>b.</b>	Inspection Personnel Housey Fellows - Pariscipal George ANION Eng.
		John F. Wallace - Georgen wich Engineer
•	e.	Persons Contacted (Including Address & Phone No.) Mr. K. Karl Mueller
		Long Pono Ct. Winamill FARAS, ARMONK, N.Y. 10504
		(914)-273-8074
,	•	
	d.	History:
•	•	Date Constructed UNKNOWN (Crea 1922) Date(s) Reconstructed
•		
		Designer Elwyne E. SEElye & Co., Chasulting Engine N.V. N.Y.
		Constructed By disknown;
•		Owner Mr. K. Yar! Mueller, Constone Ct. Hemone, N.Y.
	-	,

ankme	<u>nt</u>				
a. Characteristics					
(1)	Imbankment Material EARTH AND Rock Fill				
(2)	Cutoff Type None known to Exist				
(3)	Impervious Core None Known to EXIST				
(4)	Internal Drainage System None Known to Exist				
(5)	Miscellaneous				
	1				
Cres	st				
(1)	Vertical Alignment 9000				
(2)	Horizontal Alignment 9000				
(3)	Surface Cracks None				
(4)	Miscellaneous				
•					
Upst	ream Slope				
(1)	Slope (Estimate) (V:11)				
(2)	Undesirable Growth or Debris, Animal Burrows Dumerous bushes				
	(1) (2) (3) (4) (5) (2) (3) (4) (9) (1)				

occasional small (smaller tim 8" -1, pm) decipuous + rees

	(5)	Surface Cracks or Movement at Toe None observed
ı	Down	stream Slope
	(1)	Slope (Estimate - V:II) Broken Slope 11:15H to 11:2,0H w/20 bench 2  Here North Dorth Dorth Spillery Charried Spenden 11:3H
	(2)	Undesirable Growth or Debris, Animal Burrows Numbraus micole
	•	And (8th 18" diplocated) decicions trans- historis hends AT CACSI
	(3)	Sloughing, Subsidence or Depressions 15,110 065.65.00
	•	
	(4)	Surface Cracks or Movement at Toc ////
	•	
	(5)	Scepage sarpage or mos lose of sour neuronal - seepen
		flow Estimated to be 5to 7gpm. No soil Exosion
	(6)	External Drainage System (Ditches, Trenches; Blanket) 501/1049
		Agrical Journed on Downstron slope upac conti Acutinist.
	(7)	Condition Around Outlet Structure - one anose cutting of soiller
		5/40
	(8)	Seepage Beyond Tue None olsewed

	(1)	Erosion at Contact /////
	(2)	Scepage Along Contact <u>AT IN LOGINT OF NEST ABUTINAT</u> -  - JUANTITY 25T be be 5279pm
Drai	inage	System .
a.	Desc	ription of System NONE
	***************************************	<u> </u>
	Cond	ition of System
b	COM	
	•	harge from Drainage System / //A
	•	. ,
e. Inst	Disc	. ,
e. Inst	Disc	harge from Drainage System ///A
e. Inst	Disc	harge from Drainage System //A
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e. Inst	Disc	harge from Drainage System ///A
e. Inst	Disc	harge from Drainage System ///A
e. Inst	Disc	harge from Drainage System ///A

	Slopes Appear to be growth south - no sume of
	clausing or instruitity in Ukerenty of DAM OR SOUTH
b.	Sedimentation None
c.	Unusual Conditions Which Affect Dam Location of North Lake De
	Immeniately Hosnierm
Are	a Downstream of Dam
a.	Downstream Hazard (No. of Homes, Highways, etc.) Several Resident
	komp -
<b>b</b> .	Seepage, Unusual Growth None
	D. J. Land C. M. Land B. Dan of Dan
<b>c.</b>	Evidence of Movement Beyond Toe of Dam Nove
	, , ,
d.	Condition of Downstream Channel NATURAL SWAIL WITH fear frees
d.	Condition of Downstream Channel NATURAL SWAIL WITH fear faces  OTHERWISE GOOD
	OTHERWISE GOOD
	OTHERWISE GOOD
	OTHERWISE GOOD
	OTHERWISE GOOD  11way(s) (Including Discharge Conveyance Channel)  General Rectangualar Pennloruse Bex Colvert Courses
	OTHGRINISC GOOD  11way(s) (Including Discharge Conveyance Channel)  General Rectangualar Pennlorus Box Colvert Course  AT NORTH END of DAM - 4 high by 6 wice section 54 h
	OTHGRWISE GOOD  11way(s) (Including Discharge Conveyance Channel)  General Rectangualar Pennlorer Box Colvert Courses  AT NORTH END of DAM - 4/high by 6' wice section 54'in  length including hippeach Amo discharge 5/ABS - Walls A
	OTHGRUISE GOOD  Ilway(s) (Including Discharge Conveyance Channel)  General Rectargualar Feinforces Box Colvert Courses  AT NORTH END of DAM - 4'high by 6' wise section 54'in  length inclining Approach Amo discharge s lass - Walls a  Ift Hick with
	OTHGRAISE GOOD  Ilway(s) (Including Discharge Conveyance Channel)  General Rectangualar Perulores Box Gilvert Coentreo  AT NORTH END of DAM - 4'high by 6' wive section 54'h  Length including Appeared the discharge slass - Walls at  Ift Hick with  Condition of Service Spillway Some spalling of Terining walls and
	OTHERWISE GOOD  Ilway(s) (Including Discharge Conveyance Channel)  General Rectangualar Beinlorge Box Cilvert Coentro  AT NORTH END of DAM - 4'high by b' wive section 54'in  Length including Approach two discharge slass - Walls a  If Hick with  Condition of Service Spillway Some spalling of Texining walls and  Approach sogls - bottom slass on downstream sine is
	OTHGRUISE GOOD  Ilway(s) (Including Discharge Conveyance Channel)  General Rectargual SR Perploxes Box Colvert Courses  AT NORTH END of DAM - 4'high by 6' wise section 54'in  Length including Appeared two discharges slass - Walls as  If think with  Condition of Service Spillway Somm spalling of Teniuma walls and  Appeared sogles - bottom slass on downstrain sup is  Cracked one partially suppressed appearing 7-5 for
d. Spi a.	General Rectargualar Penulorero Box Colvert Courses  AT NORTH END of DAM - 4'high by 6' wice section 54'in  Length inclining approach Amo discharge s lass - Walls a  Ift Hick with  Condition of Service Spillway Somm spalling of Teniuma walls and

•T	c.	Condition of Auxiliary Spillway Ponousay Green Res Y bearing
		Sour ero. of Lake way horse & com charges outition feet
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		BE A SECOUPARY SAllway UNDER DENK FROM (PA)F) CONDITIONS
ì		PRINT CONDITION IS 5000- INGL SUPPORTS IN HOLIN to resel 7000.
•	d.	Condition of Discharge Conveyance Channel. Channel Section on
		DOWN STREAM DANGED to beputy PENETED WITH Suenale MASSIVE
i		Couldres - NO SIGNIFICANT ERSSION - SECTION begans toe
-	•	IN NATION Channel- Pelaquely clepic except the occasional
		decigious trees - 9000 canoitron
31	Res	ervoir Drain/Outlet
		Type: Pipe ///klown Conduit Other
		Material: Concrete Un Known Metal . Other
	- ·.	Size: UNKNOWN Length UNKNOWN
		Invert Elevations: Entrance unknown Exit unknown
		Physical Condition (Describe): Unobservable X
		Material: Misknown
		Joints: Alignment
		Structural Integrity: unknown
•		Hydraulic Capability: anknean
	-	
	•	Means of Control: Gate ? Valve ? Uncontrolled
		Operation: Operable Inoperable Other UNKNOWN
		Present Condition (Describe): Ourlet downstrain of dem is presument,
		CURRENT UNDON SECONDE POS CONDESTOS PARE POUSE IN STATES OF IN ROSESTOIRE
		· MUD WAS INTEROSOBIE

b. Structural Cracking See Nem 76  c. Movement - Horizontal & Vertical Alignment (Settlement)  See Lien 76  d. Junctions with Abutments or Embandanents spilling junctions in Chan Mrs. 1990 1012 Sincercopy of the Courses of the Course of the Courses of the Courses of the Course of the Course	
b. Structural Cracking See New 76  c. Movement - Horizontal & Vertical Alignment (Settlement)  See Tren 76  d. Junctions with Abutments or Embankments spillway junctions in Chain the Construction of the Con	
c. Movement - Morizontal & Vertical Alignment (Settlement)  See Then 76  d. Junctions with Abutments or Embankments spllwfg junctions in Offin Alice man with Abutments or Embankments spllwfg junctions in Offin Alice man with Constant in Offin Alice man were in course in Office and Institute of the Constant in Offin Annual Constant in Course where I was a constant in Offin Annual Constant in Course where the Constant in Offin Annual Constant in Course where the Cou	
c. Movement - Morizontal & Vertical Alignment (Settlement)  See Front 76  d. Junctions with Abutments or Embandments spllwpy junctions in Ohm Are the the Section of the Conservation of t	
c. Movement - Morizontal & Vertical Alignment (Settlement)  See Lien 76  d. Junctions with Abutments or Embandments spllway junctions in Ohm Are your level lives of the Course of the C	
c. Movement - Morizontal & Vertical Alignment (Settlement)  See Front 76  d. Junctions with Abutments or Embandments spllwpy junctions in Ohm Are the the Section of the Conservation of t	
d. Junctions with Abutments or Embankments spillway junctions in Chan Are case interpretation of the Course of the	<del></del>
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d. Junctions with Abutments or Embankments spillwrg junctions in  Chan Are into live liverage of the Consense  Ath Kornership luncarrace in course line  Linear to come  c. Drains - Foundation, Joint, Face NONE Observed  f. Water Passages, Conduits, Sluices	· <del></del>
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CAM ARE COST LETT SEEVENTED IN COURSE IN COURS	·
e. Drains - Foundation, Joint, Face	17/
e. Drains - Foundation, Joint, Face	an Te
e. Drains - Foundation, Joint, Face	مرکورو و ا
f. Water Passages, Conduits, Sluices	
f. Water Passages, Conduits, Sluices	
g. Scepage or Leakage leakage below downstrum sa	<del></del>
g. Scepage or Leakage leakage below downstran sa	
g. Seepage or Leakage leakage below downstream sai	
g. Seepage or Leakage leakage boles Coursing to	
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	mergy Dissipators (Plunge Pool, etc.) Nowe	
	mergy Dissipators (Plunge Pool, etc.) Nowe	
	nergy Dissipators (Plunge Pool, etc.) None.  Intake Structures NOT Obscerele	
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	nergy Dissipators (Plunge Pool, etc.) None.  Intake Structures NOT Obscerele	
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a.	Description and Co	ondition			
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	•	were	065ex	20	T STEACTURES
-				er enten annanne frankriker i størmen det gege	
*					
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HYDROLOGIC DATA AND COMPUTATIONS

# LONG POND DAM #3.

CHECK LIST FOR DAMS
HYDROLOGIC AND HYDRAULIC
ENGINEERING DATA

#### AREA-CAPACITY DATA:

. '	•	Elevation * (it.)	Surface Area (acres)	Storage Capacity (acre-ft.)
1)	Top of Dam	476	11.28	173.6
2)	Design High Water (Max. Design Pool)	Miskugur		
	Auxiliary Spillway . Crest	Low highway en	nbenkines	
4)	Pool Level with Flashboards			
5)	Service Spillway Crest	470	.826	115

#### DISCHARGES

•			•	(cis)
1)	Average Daily	•		Unknown
2)	Spillway @ Maximum High Water		•	222
3)	Spillway @ Design High Water			MUKNOEN
4)	Spillway @ Auxiliary Spillway Crest	Elevation	١	1/11 NOWN
5)	Low Level Cutlet	•		UNKNOSUN
6)	Total (of all facilities) @ Maximum	ı Niğh Wate	r	222=
7)	Haximum Known Flood			phyhomu
8)	At Time of Inspection			NUK HOWN

\* All Elevations ALE Gaseo on Peno Level Datum.

Velative to antelevation interpolace Thom the USAS Norse Lale, NY.

QUAD Let.

Eniber Land CREST:	, 1.1	· EL	EVATION:	476
Type: En-	- 11 to Kacl= fill			
	34'	Length:	365	· .
\$pillover	NONE.	-		
· •				
SPILLWAY:	•			•
SERVIC	C <del>/</del>		AUXILI	ARY
410	INVERT Elev	vation		
40 HX 6.0 W	box culvert Ty			
Approx 54.	o'intenation wie	ith		
	Type of (	Control	•	÷
·	Unconti	rolled		
	Contro	olled:		
	T	уре	*******************************	
	(Flashboard	s; gate)		•
	Numbe	er ` <u> </u>	• · · · · · · · · · · · · · · · · · · ·	
	Size/Lo	ength \	······································	
	Invert Ma	terial		
•	Anticipated of operating	_		
•	Chute L	ength	·	
		Spillway Cres hannel Invert Flow)		

урс:	
ocation:	<u> </u>
Records:	
Date -	
Max. Reading  D WATER CONTROL SYSTEM: Warning System:	None
Method of Controlled Re	

				.0.52	
INAGE	BASIN RUNOFF C	HARACTERISTICS:	0		
	Use - Type: _	W0:55 -		+# BIVE	
Terra	in - Relief: _	· Hirry			
\$urfa	ce - Soil:	GLACIAL	TILL		
Runof		existing or plann			existing
. •	(S	surface or subsur	tace condition		
	•	In pure	9.03.100	<u> </u>	
•			<del></del>		
		No			
Poten		problem areas f charge storage:	or levels at m		capacity
Poten		r problem areas f			capacity
Poten		r problem areas f	or levels at m		capacity
Poten		r problem areas f	or levels at m		capacity
	including sur	problem areas freharge storage:  (overflow & non-	None:	naximum storage	
	- Floodwalls Reservoir per	problem areas freharge storage:  (overflow & non-	or levels at n  Nonc.  overflow ) - L	naximum storage	
	- Floodwalls Reservoir per	problem areas frehange storage:  (overflow & non-imeter:	or levels at n  None.  overflow ) - L	naximum storage	
Dikes	- Floodwalls Reservoir per	problem areas freharge storage:  (overflow & non-imeter:	or levels at n  None.  overflow ) - L	naximum storage	
Dikes	- Floodwalls Reservoir per Location: Elevation:	problem areas frehange storage:  (overflow & non-imeter:	or levels at n  None,  overflow ) - L	ow reaches alo	ng the

Project	1579-10  Long Poid DAY NO Z  Hypeologic / Hypeologic Compositions  Hypeologic / Hypeologic Compositions  Hypeologic Vind Cose No Silosopp Chik.by  Chik.by
	Assume.
	1. BASKU LIPSTREAM OF WINDMILL LAKE IS COMPLETELY CONTROLLED & WILL NOT CONTRIBUTE to LONG POND INFL.
	2. For Analysis Basin is divided into 3 Sub-areas
	(1) North-Lace sub-acce.
	(a) Sub-account downstroom of North boke.
	3) Subance . 2 West et Long Pene mining de Windmine FART TOTAL AFER = 0.46 STATILLES.
<i>ب</i> د	Jue. Arta 1. (59.2 ac + 0.09 mi)
	L = 1900 = 0.30 miles
	Lax 2 400' . (1.676 miles
	Use C, = 2 of 640 Cp : 400 Cp : 0.625
	to = 2 (136 × 0.076) = 0 68 hrs
	£ = 00% = 0.12 hrs
	FOR tR = 0.53 hrs
	tpr tp + 0.25 (0.35 - 0.12) = 0.73 hours. % improveds 4.13/59.2 = 0.07
S	UE. AREA 2 (67.5 Dans = 0. 11 mi2)
	L = 1600' = 0.3 mile
	to, 600' + 0 11 mile
U:	C1 = 2 A GACC, - 400
	to 2 (0.280 H) = 0.72 h
	ty + in the contraction

Job No. 1579-10 Project LONG POND DAM #3	Sheet 2 of 35
Subject Hydrologic/Hydrouic Comprisions	By
	Ch'k. by

SUB AREA 2 con- % imperious  $\frac{4.13}{67.5}$  c.06  $t_{PR} = t_{P} + 0.25(t_{R} - t_{A})$ FOR  $t_{R} = 0.33$  hrs.  $t_{PR} = 0.72 + 0.25(0.33 - 6.13) = 0.77$  hours

NORTH LAKE Sub-bosin (from North Line DAN #3)

tpr = 0.98 hrs tr = 0.83 hrs

Job No. 1579-10

Project Long Ponis DAM No 3:

Sheet 3 of 85

Date Jule 18, 81

Subject Hypraulic Computation By D.L.C.

Ch'k, by HMD.

SPILLWAY RATING INVERT EL 470. USE MANDING FORMULA for deptiful= 1, 2, 35 Q. 109- ARTOSTA 5 = 0.0025 h = 0.01.5 P Mp Ky cl .75 2.4.6 - 3 99.3 6. 8 .65 2 67.3 1.2 12 10 .01 3.5 1.6 142.6 2.1 13 **(5** 6.6 d - 4. Q1 1 ( See attracted chart, Open -Hid H CHEMONER HYSOPULIES - CHOW) 24-0.3 1.2 1471-2 0.5 2.0 3.8 53 472 1.0 45 b 2.5 474 the of war interest. 1000 1.25 · 6 30 475 1. 375 275.5 5.5 33.5 201 476 1.5 TOP OF DAM EL (-57 272 2.0 478 8 270 53 2.5 10 518 490

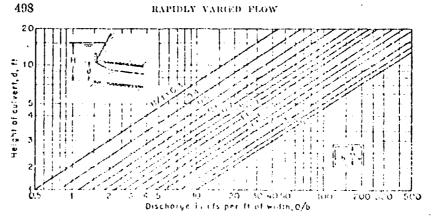
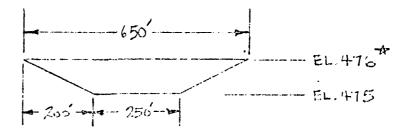


Fig. 17-29. Chart for estimating braidwater on box cuts cits with square edged entrances, flowing partly full. (Has 2 on nature U.S. Pen, we of P.2 on Rouds (2.9))

Joh No. 1579-10

Project Love Power Date No. 8

Subject Love Upot in Arrenage Struck Struck Ct. By Struck by Secondary Spillingary - Discharge No. 112 VATION Ch'k, by



ASSUME: 4:0.025 (LANN & ROADWAY SURFACES)

ELEV.	A (GE)	<u>.</u>	B.	825 GG.	O= 1.49 AR 5/2=1.9072 AR
475					0
475.5	175	45c	0.39	0.55	177
4-76	450	650	0.69	0.70	669
478	1750	65H	2.68	1.43	6423

FARTH ELL HIR, THE SPILLWAY WITH WILL REMAIN AT 650 FT.

NOTTH LANG (IDANE) - GODPACHE:

ELECTIVE ELLE	44	Ass. A	YE AN AMA	A Ver	STORFOE AG-17.	to prompt to error under pure a la description.
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574	1	22.9	23.75	23.25	24.03	
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577	7.	25.0	14.3	1 48.6 1	301.25	
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092		27,1		i	400.5	

Job No. 1577-15	Sheet 5 of CF
Project 6. 13. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2.	Date
Subject	By
	Ch'k. by

Loug Pond	- SPILLION Y	(SE Corner of tale)	
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471.2	2.4	O	24
472.	53	0	53
414	150	٥	150
475	186	Ö	185
405 8	201	. 177	378
476 .	Con Con	669	ଌ୩ ।
4 V4 4 7 8	270	6.43B	6703 445

5	PILLWAY DISCHARCE	CAPACITY	BREADTH	≈ 6 o'
1	Mestar (Nutry Lak		2·5 p	
Q=CLH3/	7.0' 85	3.0	7.0'	
E.L.	Li H Ci Q	L H C	$C^{(r)}$	Q,
572	3 0 0	0 0		0
573	3 1 2.68 8	14 0.5 2.6	129	20.9
573.75	3 175 2 65 18.4	14 1.25 2.60	52.0	70.4
575.	3 3 266 11.5	14 2 5 267	147.8	189.3

Job No. 1579-10

Project Long Ponjo Dam No Z

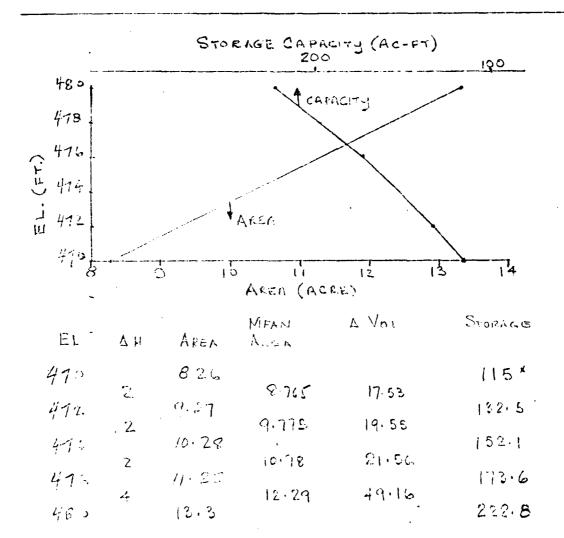
Sheet G of ST

Date June 18

Subject Hydrologic Hydraug Computation

By 1922

Ch'k. by



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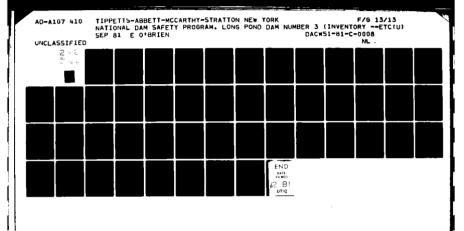
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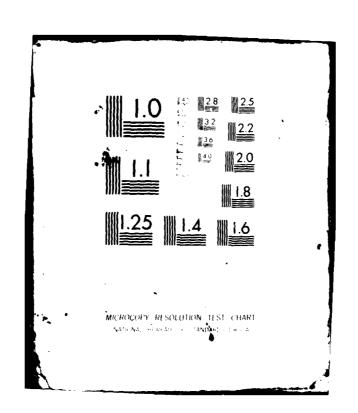
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APPENDIX E

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